

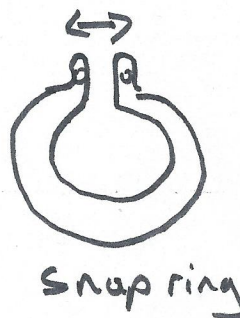
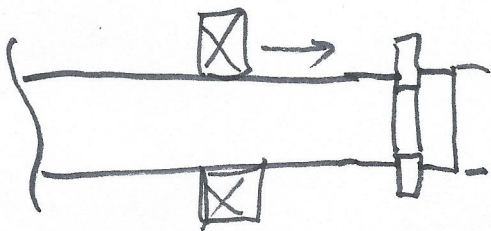
Deformation and Stiffness Chap 4

Design for high rigidity

- minimize misalignment
- avoid interference w/ other components
- reduce noise
- reduce wear rates
- reduce stress

Design for flexibility

- energy storage and absorption
  - springs
- elastic deformations for change in dimensions

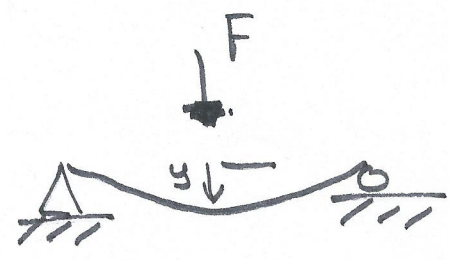
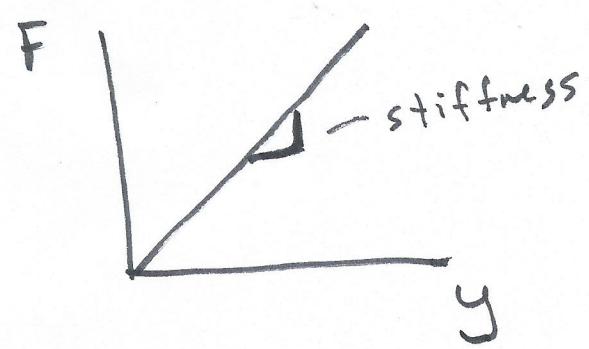


Snap ring

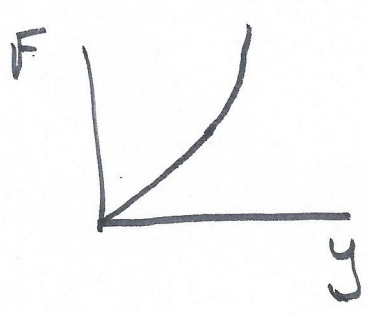
Rigidity deflection per load  $\frac{\Delta y}{F}$

- mod of Elasticity gives a good indication of rigidity
- geometry of the structural element is essential to characterize rigidity
- inverse of rigidity is "stiffness"  
 $k = \frac{F}{\Delta y}$

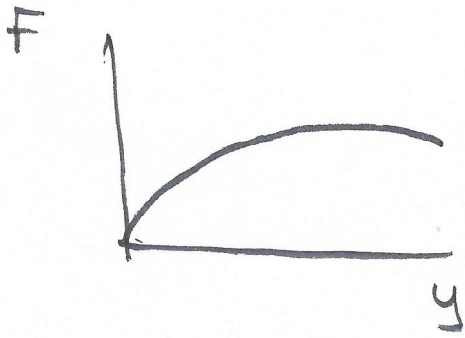
Linear Springs



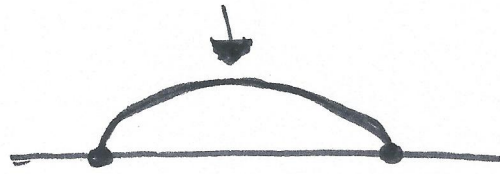
Nonlinear Springs



stiffening spring



Softening spring



Stiffness (spring constant) for linear springs

Axial

$$\delta = \frac{Fl}{AE}$$

↓ stiffness

$$k = \frac{AE}{l}$$

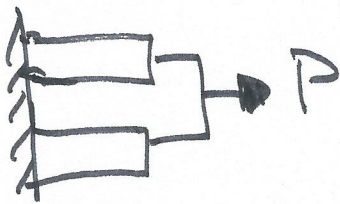
Torsional

$$\theta = \frac{Tl}{GJ}$$

$$k = \frac{GJ}{l}$$

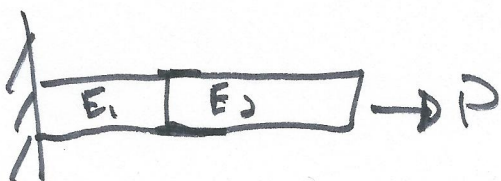
↗ torsional rigidity

Parallel springs



$$k = \sum_{i=1}^N k_i$$

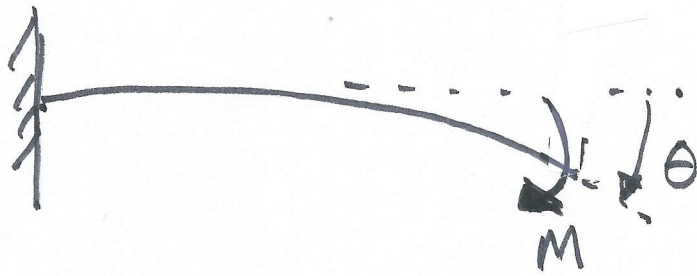
Series



$$k = \frac{1}{\sum_{i=1}^N \frac{1}{k_i}}$$

Bending stiffness due to (angular stiffness)

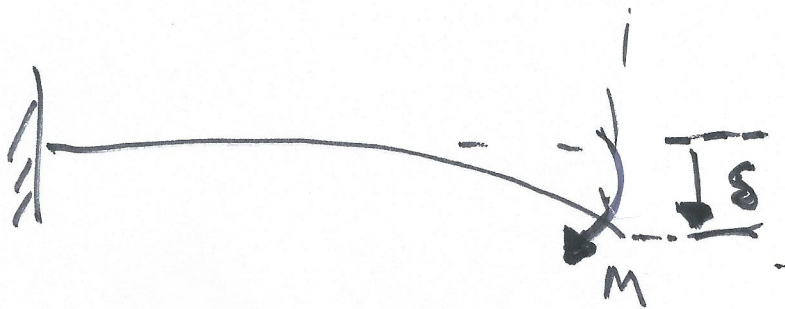
Moment that cause angular def. flexural rigidity



$$\theta = \frac{ML}{EI}$$

$$k = \frac{EI}{L}$$

Bending (linear stiffness)



$$\delta = \frac{ML^2}{2EI}$$

$$k = \frac{2EI}{L^2}$$

Bending (linear stiffness w.r.t Force)



$$\delta = \frac{P \cdot L^3}{3EI}$$

$$k = \frac{3EI}{L^3}$$

# Relative Stiffness

① 1" OD round steel bar, 10" w/ tensile load

$$k = \frac{AE}{l} \Rightarrow k = 23.6 \times 10^5 \frac{\text{lb}}{\text{in}}$$

② same element w/ bending

$$k = \frac{3EI}{l^3} \Rightarrow k = 4.4 \times 10^3 \frac{\text{lb}}{\text{in}}$$

③ cantilever w/ shear load

$$k = \frac{9AG}{10l} \Rightarrow k = 81.3 \times 10^4 \frac{\text{lb}}{\text{in}}$$

same bar is most stiff axially, then in shear, and then in bending

④ torsion (solid bar)

$$k_t = \frac{JG}{l} \Rightarrow k_t = 1.13 \times 10^5 \frac{\text{lb}\cdot\text{in}}{\text{rad}}$$

⑤ tube w/ circular cross section same area as the bar in ①

$$k_t = \frac{JG}{l} \Rightarrow k_t = 55.3 \times 10^5 \frac{\text{lb}\cdot\text{in}}{\text{rad}}$$

tube with same amount of material is 55X stiffer

# Deflection of Beams

theory of elasticity

curvature:  $\frac{1}{\rho} = \frac{M}{EI}$

From math:

rho

$$\frac{1}{\rho} = \frac{d^2y/dx^2}{\left[1 + \left(\frac{dy}{dx}\right)^2\right]^{3/2}}$$

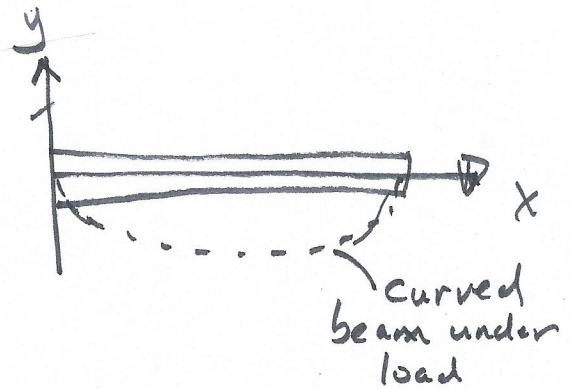
⇓

$$\frac{1}{\rho} = \frac{d^2y}{dx^2}$$

$$\boxed{\frac{M}{EI} = \frac{d^2y}{dx^2}}$$

$$\frac{V}{EI} = \frac{d^3y}{dx^3}$$

$$\frac{q}{EI} = \frac{d^4y}{dx^4}$$



$\frac{dy}{dx}$  is small

$$\frac{dy}{dx} \approx 0$$

Given  $q(x)$

$$\left. \begin{aligned} V &= \int q(x) dx + C_v \\ M &= \int V(x) dx + C_m \end{aligned} \right\} \begin{array}{l} C_v \text{ and } C_m \\ \text{obtained} \\ \text{from static} \\ \text{equilibrium conditions} \end{array}$$

$$\left. \begin{aligned} EI \Theta &= \int M(x) dx + C_1 \\ y &= \int \Theta(x) dx + C_2 \end{aligned} \right\} \begin{array}{l} C_1 + C_2 \\ \text{obtained from} \\ \text{boundary conditions} \end{array}$$

Multiple Ways to solve these

- method of sections **S**
- superposition \*
- moment area method
- singularity functions \*
- numerically integrate

# Superposition

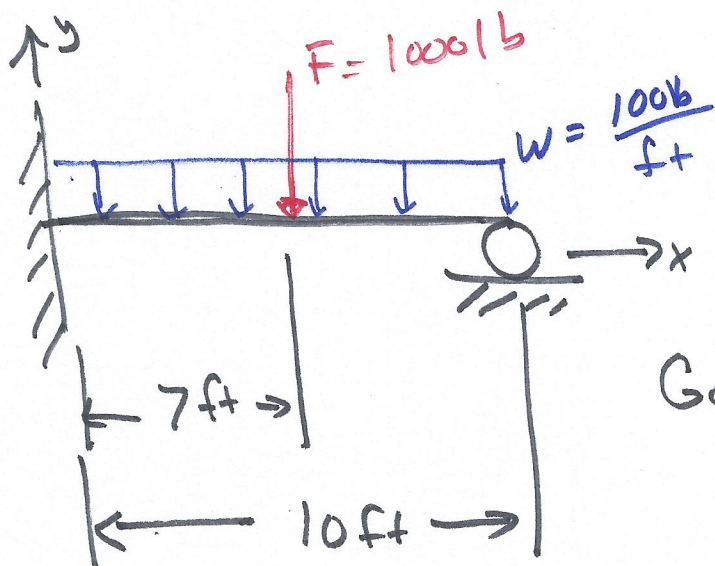
- many simple beam cases have been pre-solved and the results can be algebraically combined
- Table A-9  $\Rightarrow$  look for small set of beams
- Roark's Formulas for Stress and Strain

## Rules

- each effect must be linearly related to the load that produced it, e.g.  $\delta = \frac{PL}{AE}$
- a load does not create a condition that affect the result of another load
- deformation from any specific load is not large enough to appreciably alter the geometric relation of the structural system



# Example: Superposition



$$E = 30 \times 10^6 \text{ psi}$$

$$I = 5 \text{ in}^4$$

Goal: What is  $y_{\max}$ ?

From Table A-9 #12

$$y_{AB} = \frac{Fbx^2}{12EI l^3} \left[ 3l(b^2 - l^2) + x(3l^2 - b^2) \right]$$

$$y_{BC} = y_{AB} - \frac{F(x-a)^3}{6EI}$$

#13

$$y = \frac{wx^2}{48EI} (l-x)(2x-3l)$$

$$\left. \begin{aligned} y_{\text{tot } AB} &= y_{AB} + y \\ y_{\text{tot } BC} &= y_{BC} + y \end{aligned} \right\} \text{superposition}$$

$$y_{\text{total AB}} = c_1 x^4 + c_2 x^3 + c_3 x^2 + c_4 x' + c_5$$

$$\frac{dy_{\text{total AB}}}{dx} = 0 \quad \text{Solve for } x \text{ where } \Theta = 0$$

▲ cubic polynomial

See the Jupyter Notebook with code to solve for the roots of this polynomial.

\* Note: determine if max deflection is between  $0 \leq x \leq 7$  or  $7 \leq x \leq 10$  \*

$$\Theta_{AB}(x) = -9.26 \times 10^{-9} x^3 + 3.54 \times 10^{-6} x^2 - 2.1 \times 10^{-4} x$$

$$\Theta_{BC}(x) = -9.26 \times 10^{-9} x^3 + 2.05 \times 10^{-7} x^2 + 3.5 \times 10^{-4} x - 0.024$$

$\Theta_{BC} = 0 \Rightarrow$  No solutions between 7ft and 10ft

$$x = (0, 73.11, 309.03) \text{ in for } \Theta_{AB}(x) = 0$$

only valid answer

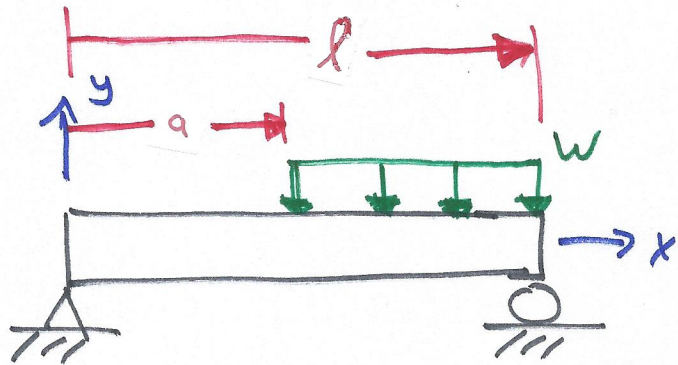
$$y_{AB}(73.11) = \boxed{y_{\text{max}} = -0.102''}$$

# Deflection of Beam : Singularity Functions

$$EI\theta = \int M(x) dx + C_1 \Rightarrow \theta = \frac{1}{EI} \int M(x) dx + C_1$$

$$y = \int \theta(x) dx + C_2 = \frac{1}{EI} \iint M(x) dx + C_3$$

Example



Find

1) expressions for slope and deflection  
and also 2) max deflection with following parameters

$$l = 10''$$

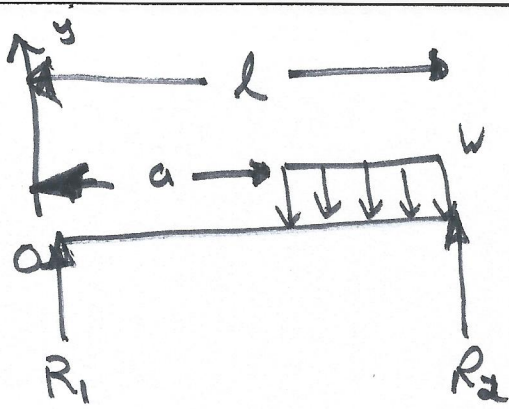
$$a = 4''$$

$$w = 100 \frac{\text{lb}}{\text{in}}$$

$$E = 30 \text{ Mpsi}$$

$$I = .163 \text{ in}^4$$

FBD



$$q(x) = R_1 \langle x \rangle^{-1} - w \langle x-a \rangle^0 + w \langle x-e \rangle^0 + R_2 \langle x-l \rangle^{-1}$$

$$V(x) = R_1 \langle x \rangle^0 - w \langle x-a \rangle^1 + w \langle x-e \rangle^1 + R_2 \langle x-l \rangle^0$$

$$M(x) = R_1 \langle x \rangle^1 - \frac{w}{2} \langle x-a \rangle^2 + \frac{w}{2} \langle x-e \rangle^2 + R_2 \langle x-l \rangle^1$$

$$V(l^+) = 0 = R_1 - w(l-a) + w(l-e) + R_2$$

$$M(l^+) = 0 = R_1 l - \frac{w}{2} (l-a)^2 + \frac{w}{2} (l-e)^2 + R_2 (l-e)$$

*to small*

$$\underline{R_1 = \frac{w}{2l} (l-a)^2}$$

$$0 = \frac{w}{2l} (l-a)^2 - w(l-a) + R_2$$

$$\underline{R_2 = w(l-a) - \frac{w}{2l} (l-a)^2}$$

$$R_1 = 180 \text{ lb} \quad R_2 = 420 \text{ lb}$$

\* drop discontinuities @  
end of beam \*

$$EI\theta = \frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 + C_1$$

$$\theta = \frac{1}{EI} \left[ \frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 + C_1 \right]$$

$$EI y = \frac{R_1}{6} \langle x \rangle^3 - \frac{W}{24} \langle x-a \rangle^4 + C_1 x + C_2$$

$$y(0) = 0$$

$$y(l) = 0$$

no deflection  
@ simply supported  
ends

boundary conditions

$$\text{@ } x=0: 0 = C_2$$

$$\text{@ } x=l: 0 = \frac{R_1 l^3}{6} - \frac{W}{24} (l-a)^4 + C_1 l$$

Solve for  $C_1$  and  $C_2$

$$C_1 = \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6}$$

$$C_2 = 0$$

1) Expressions for  $\theta$  and  $y$ :

$$\theta = \frac{1}{EI} \left[ \frac{R_1}{2} \langle x \rangle^2 - \frac{W}{6} \langle x-a \rangle^3 + \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6} \right]$$

$$y = \frac{1}{EI} \left[ \frac{R_1}{6} \langle x \rangle^3 - \frac{W}{24} \langle x-a \rangle^4 + \frac{Wx}{24l} (l-a)^4 - \frac{R_1 l^2 x}{6} \right]$$

The max deflection will be where  $a < x < l$ , so:

$$\Theta(x) = \frac{1}{EI} \left[ \frac{R_1}{2} x^2 - \frac{W}{6} (x-a)^3 + \frac{W}{24l} (l-a)^4 - \frac{R_1 l^2}{6} \right]$$

Where is  $\Theta(x) = 0$ ? Solve cubic polynomial for  $x$ .

$$-3.4E-6x^3 + 5.93E-5x^2 - 1.64E-4x - 2.95E-4 = 0$$

$$x = (-1.19, \underline{5.26}, 13.3)$$

2) max deflection  $4'' < 5.26'' < 10''$  only valid answer

$$y(5.26) = -0.00176''$$